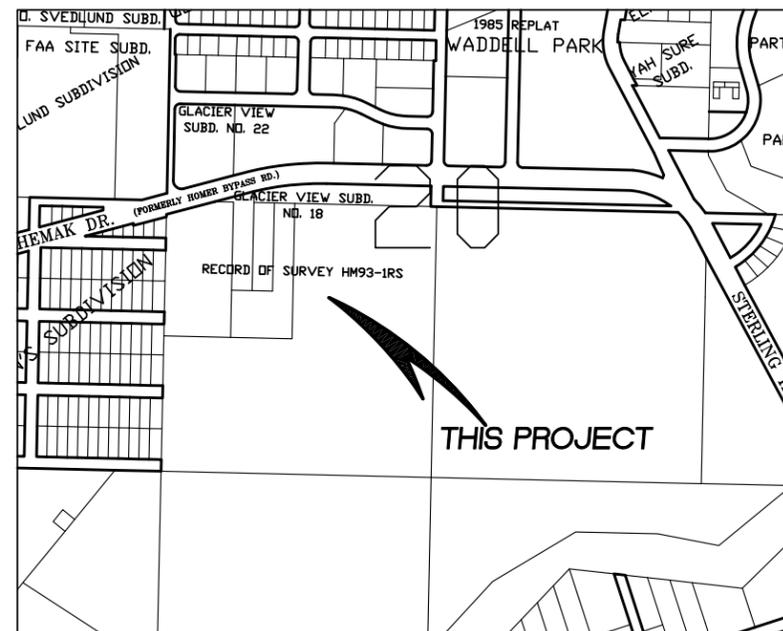


CITY OF HOMER PUBLIC WORKS EQUIPMENT STORAGE BUILDING

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VICINITY MAP

GENERAL NOTES

2009 IBC CODE DATA	
OCCUPANCY GROUP	S-2
CONSTRUCTION TYPE	V-B
ALLOWABLE AREA	1 STORY - 13,500SF
ACTUAL AREA	1 STORY - 2400SF
OCCUPANT LOAD	5
EXITING	1 EXIT REQUIRED*
LEGAL DESCRIPTION: KENAI PENINSULA BOROUGH PARCEL # 17714016	
*NOTE: ALL BAYS ARE OPEN ON ONE SIDE	



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- 1.) LOCATION OF UNDERGROUND UTILITIES ARE APPROXIMATE. ACTUAL DEPTH, NUMBER AND LOCATION UNKNOWN. BURIED UTILITIES OTHER THAN THOSE SHOWN ON THE PLANS MAY BE PRESENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATION AND IDENTIFYING ALL UTILITIES. CALL FOR LOCATES PRIOR TO EXCAVATION.
- 2.) THE FOLLOWING ALASKA STATUTES APPLY TO WORK NEAR OVERHEAD ELECTRIC LINES:
 - AS 18.60.670 (1)
PLACEMENT OF AN TYPE OF TOOL, EQUIPMENT, MACHINERY OR MATERIAL THAT IS CAPABLE OF LATERAL, VERTICAL OR SWINGING MOTION, WITHIN 10' OF ENERGIZED LINES IN NOT ALLOWED.
 - AS 18.60.670 (2)
MINIMUM 10' CLEARANCE FROM BUILDINGS, APPARATUS, MACHINERY, MATERIALS, ETC.
 - AS 18.60.680
ANY WORK WITHIN MINIMUM DISTANCE STATED ABOVE SHALL REQUIRE CONTACT WITH HEA TO INSTALL TEMPORARY MECHANICAL BARRIERS, TEMPORARY DE-ENERGIZATION AND GROUNDING, OR TEMPORARY RAISING OF CONDUCTORS.
- 3.) ALL WORK SHALL CONFORM TO CITY OF HOMER STANDARD SPECIFICATIONS.

GENERAL

ALL MATERIALS AND CONSTRUCTION SHALL CONFORM TO REQUIREMENTS OF THE INTERNATIONAL CODE COUNCIL INTERNATIONAL BUILDING CODE (IBC) 2009 EDITION. WHERE EXPLICIT DETAILS ARE NOT SHOWN OR DESCRIBED, THE MINIMUM REQUIREMENTS OF THE ABOVE CODE SHALL APPLY. UNLESS OTHERWISE NOTED, ALL CODES, STANDARDS AND OTHER PUBLICATIONS CITED SHALL REFER TO THE LATEST EDITION.

LOCATION

PROJECT IS LOCATED IN HOMER, ALASKA

DESIGN LOADS

IN ADDITION TO DEAD LOADS, THE FOLLOWING LIVE LOADS WERE USED FOR DESIGN:

ROOF: 50 P.S.F. (SNOW-2 MONTH DURATION) PLUS DRIFTING WIND: 120 MPH, EXPOSURE D
SEISMIC: Ss=1.5, S1=0.5, SITE CLASS D.

LATERAL LOADS ARE RESISTED BY WOOD SHEAR WALLS AND DIAPHRAGMS.

SITE WORK

PREPARATION OF A SAFE AND SUITABLE BUILDING SITE SHALL BE COMPLETED PRIOR TO CONSTRUCTION OF FOUNDATIONS AND SLABS. NO SITE INVESTIGATION OR SOILS BORINGS WERE PERFORMED BY THE ENGINEER. THE FOUNDATION DESIGN IS BASED ON THE ASSUMPTION THAT SOILS BENEATH THE FOUNDATION ARE WELL DRAINED NON FROST SUSCEPTIBLE SAND OR GRAVEL WITH LOAD CAPACITIES IN ACCORDANCE WITH IBC TABLE 1804.2 ALLOWABLE BEARING CAPACITY OF 2000 PSF WAS USED FOR DESIGN.

SITE PREP FOR FOUNDATIONS

BUILDING WILL BE CONSTRUCTED ON AN EXISTING GRAVEL FILL PAD. GRADE PAD SMOOTH AND TO DRAIN AWAY FROM BUILDING INTERIOR PRIOR TO PLACING HELICAL PILE FOUNDATIONS.

HELICAL PILE FOUNDATIONS

A.1 BEARING PILE PERFORMANCE REQUIREMENTS:

1. DRIVE PILES TO MINIMUM DEPTHS INDICATED.
2. MINIMUM ULTIMATE PILE BEARING CAPACITY: 30,000 POUNDS;
3. MINIMUM ALLOWABLE PILE BEARING CAPACITY: 15,000 POUNDS
4. MINIMUM ALLOWABLE PILE TENSION CAPACITY: 15,000 LBS, INCLUDING COUPLINGS.

A.2 WIND PILE PERFORMANCE REQUIREMENTS:

1. DRIVE PILES TO MINIMUM DEPTHS INDICATED.
2. MINIMUM ULTIMATE PILE BEARING CAPACITY: 5,000 POUNDS;
3. MINIMUM ALLOWABLE PILE BEARING CAPACITY: 2,500 POUNDS
4. MINIMUM ALLOWABLE PILE TENSION CAPACITY: 2,500 LBS, INCLUDING COUPLINGS.

B. PROVIDE PILE DRIVING RECORDS INCLUDING:

1. SIZE, LENGTH, AND LOCATIONS OF PILES.
2. SEQUENCE OF DRIVING.
3. INSTALLATION TORQUE REQUIRED AND TORQUE CORRELATION FACTOR (Kt) FOR THE PILE.
4. IDENTIFY DRIVING CONDITIONS FOR EACH PILE INCLUDING OBSTRUCTIONS OR OTHER ANOMALIES.
5. FINAL PILE TIP AND TOP ELEVATIONS

C. PROVIDE STEEL MILL CERTIFICATIONS FOR COMPLIANCE, AND TEST RESULTS.

D. PILES AND PILE CAP MATERIAL:

1. TECHNO METAL POST- P3 HELICAL PILE WITH 3/4" SCHEDULE 40 PIPE SHAFT AND 1/2" THICK PLATE. HELIX SHALL BE AS REQUIRED TO ACHIEVE SPECIFIED LOAD CAPACITY. ALL HELICAL PIERS SHALL BE PROVIDED WITH A 8" LONG X .10" THICK POLYETHYLENE FROST PROTECTION SLEEVE. PROVIDE SUBMITTAL TO ENGINEER FOR APPROVAL PRIOR TO SUBSTITUTION.
2. LEAD AND EXTENSION SHAFT SHALL BE ASTM A 500 GRADE C PIPE, 46 KSI MINIMUM YIELD. LEAD AND EXTENSION SHAFTS SHALL BE FURNISHED IN 7' OR 10'-6" LENGTHS. EXTENSION SHAFT SHALL BE CONNECTED TO LEAD SHAFT WITH 4" OD X .226" WALL X 3 1/2" LONG WELDED OR BOLTED COUPLER. COUPLERS SHALL BE FASTENED WITH 1/2" CONTINUOUS FILLET WELD AT TOP AND BOTTOM OF COUPLER OR WITH (2) 3/8" DIA MACHINE BOLTS AND NUTS.
3. HELICAL PLATE: THICK ASTM A36 HOT ROLLED STEEL PLATE.
4. SUPPORTING PLATE: ASTM A36 HOT ROLLED STEEL PLATE.
5. HOT DIPPED GALVANIZED AFTER FABRICATION: PER ASTM A-153.
6. WELDING: AMERICAN WELDING SOCIETY D1.1 E70 ELECTRODES. CSA STANDARD W47.1. COAT FIELD WELDS WITH BRUSH APPLIED ZINC RICH PAINT.
7. FASTENERS: ALL BOLTS SHALL BE ASTM A 325, HOT DIPPED GALVANIZED.

G. TOLERANCES:

1. VARIATION FROM VERTICAL: 1 IN 200.
2. TOP CUT OFF ELEVATION: MAXIMUM 0.125 INCH.
3. HORIZONTAL LOCATION: MAXIMUM 1.0 INCH.

I. UNACCEPTABLE PILES:

1. PILES THAT FAIL TESTS, ARE PLACED OUT OF POSITION, BELOW CUTOFF ELEVATION, DAMAGED, OR THAT DO NOT MEET THE MINIMUM BEARING CAPACITY REQUIREMENT.
2. PROVIDE ADDITIONAL PILES OR REPLACE PILES TO CONFORM TO SPECIFIED REQUIREMENTS.

SAWN LUMBER AND TIMBER

LUMBER SHALL CONFORM TO THE CLASSIFICATION, DEFINITION, AND GRADING REQUIREMENTS OF IBC CHAPTER 23 WITH ALLOWABLE UNIT STRESSES AS GIVEN IN THE AMERICAN FOREST & PAPER ASSOCIATION 'NATIONAL DESIGN SPECIFICATION 2001 SUPPLEMENT', TABLE 4A. LUMBER SHALL BE GRADE MARKED BY THE WEST COAST LUMBER INSPECTION BUREAU /WESTERN WOOD PRODUCTS ASSOCIATION.

ITEM	SPECIES
4 X AND LARGER	DOUGLAS FIR #2 EXCEPT WHERE DF#1 NOTED ON DRAWINGS
WALL GIRTS	HEM FIR #2
ALL OTHER LUMBER	HEM FIR #2

ALL LUMBER SHALL BE FASTENED IN CONFORMANCE WITH TABLE 2304.9.1 OF THE IBC, UNLESS NOTED OTHERWISE. FASTENERS SHALL BE GALVANIZED UNLESS OTHERWISE NOTED. FASTEN ALL JOIST BLOCKING TO PLATES WITH (4) 16D MINIMUM AND FASTEN ALL WALL PLATES TO WOOD FLOORS WITH 16D AT 6" ON CENTER TYPICAL. DOUBLE TOP PLATES SHALL OVERLAP 10' - 0" MINIMUM AND SHALL BE SPLICED TOGETHER WITH 16D NAILS AT 6" ON CENTER MINIMUM UNLESS NOTED OTHERWISE.

PROVIDE JOIST/BEAM HANGERS WITH LOAD CAPACITY EQUAL TO SUPPORTED MEMBER SHEAR LOAD CAPACITY FOR ALL MEMBERS NOT OTHERWISE PROVIDED WITH DIRECT BEARING SUPPORT. PROVIDE A MINIMUM OF (2) KING STUDS AND (2) CRIPPLE STUDS FOR ALL BEARING WALL HEADERS. PROVIDE A MINIMUM OF (1) KING STUD AND (1) CRIPPLE STUD AT NON-BEARING WALL HEADERS. PROVIDE SOLID BLOCKING SUPPORT FOR BEAMS AND HEADERS CONTINUOUS DOWN TO FOUNDATIONS. MINIMUM HEADER OVER OPENINGS IN BEARING WALLS SHALL BE 4X12 DF#1 UNLESS NOTED OTHERWISE. MINIMUM HEADER IN NON-BEARING INTERIOR PARTITION WALLS SHALL BE A SINGLE 2X8.

BOLT HEADS AND NUTS BEARING AGAINST WOOD TO BE PROVIDED WITH FLAT WASHERS. SOLID BLOCKING OF NOT LESS THAN 2" NOMINAL THICKNESS SHALL BE PROVIDED AT ENDS AND AT ALL SUPPORTS OF JOISTS AND RAFTERS, UNLESS SHOWN OTHERWISE. BEAM AND JOIST HANGERS SHALL HAVE A CAPACITY EQUAL TO THE SHEAR STRENGTH OF THE BEAM OR JOIST WHICH IT IS SUPPORTING, UNLESS NOTED OTHERWISE. ALL METAL FRAMING ANCHORS AND HANGERS SHOWN ON DRAWINGS SHALL BE "STRONG TIE CONNECTORS" AS MANUFACTURED BY SIMPSON COMPANY OR APPROVED EQUAL. ALL SIMPSON CONNECTORS IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE TYPE 304 OR TYPE 316 STAINLESS STEEL.

PRESSURE TREATED WOOD

ALL WOOD INDICATED AS PRESSURE TREATED, (PT) SHALL BE PRESSURE TREATED (PT) IN ACCORDANCE THE AMERICAN WOOD PRESERVER'S ASSOCIATION STANDARD U1-02. THE PRESERVATIVE SHALL BE ALKALINE COPPER QUAT (ACQ). ALL WOOD SHALL BE TREATED TO A RETENTION OF 0.60 PCF AS REQUIRED FOR 'GROUND CONTACT' ALL PRESSURE TREATED WOOD SHALL BE APPROPRIATELY MARKED ATTESTING TO COMPLIANCE WITH THESE REQUIREMENTS. LUMBER SHALL BE DRIED AFTER TREATMENT TO A MOISTURE CONTENT OF 19% OR LESS. ALL BOLTS, NAILS AND SIMPSON CONNECTORS IN CONTACT WITH PRESSURE TREATED WOOD BELOW GRADE SHALL BE TYPE 304 OR TYPE 316 STAINLESS STEEL.

GLUED LAMINATED STRUCTURAL UNITS

MATERIALS, MANUFACTURE, AND QUALITY CONTROL OF GLUED LAMINATED STRUCTURAL UNITS (GLULAM) SHALL CONFORM TO IBC CHAPTER 23 WITH ALLOWABLE STRESSES AS DEFINED IN AMERICAN FOREST & PAPER ASSOCIATION 'NATIONAL DESIGN SPECIFICATION 2001 SUPPLEMENT', TABLE 5A, AND SHALL BE DOUGLAS FIR, COMBINATION 24F-V8, DF/DF. GLUE LAMINATED STRUCTURAL UNITS SHALL BE GRADE MARKED BY THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION, AITC.

PLYWOOD

ALL PLYWOOD SHALL CONFORM TO UBC STANDARD 23-2 AND SHALL BE AMERICAN PLYWOOD ASSOCIATION GRADE TRADE MARKED. PLYWOOD SHALL BE GROUP I OR GROUP II DOUGLAS FIR. ALL PANELS SHALL BE NOMINAL 4' X 8' PANELS. UTILIZE FULL SHEETS WHEREVER POSSIBLE. LAY FACE GRAIN OF ROOF AND FLOOR SHEATHING PANELS PERPENDICULAR TO JOISTS AND WITH PANEL CONTINUOUS OVER THREE OR MORE SPANS. STAGGER END JOINTS OF SUCCESSIVE COURSES 4' - 0". WALL SHEATHING SHALL BE INSTALLED WITH THE FACE GRAIN PARALLEL TO STUDS, (LONG DIMENSION VERTICAL).

ROOF SHEATHING: SHALL BE 5/8" THICK GRADE APA 40/20 SPAN RATED PLYWOOD WITH EXTERIOR GLUE. ROOF SHEATHING SHALL BE FASTENED TO END SUPPORTS WITH 10D GALVANIZED NAILS (0.148"x3") AT 6" O/C. AT BLOCKED DIAPHRAGM LOCATIONS, FASTEN PLYWOOD TO FRAMING AT ALL PANEL EDGES WITH 10D GALVANIZED NAILS @ 4" O/C. AT ALL LOCATIONS, FASTEN PLYWOOD TO INTERMEDIATE SUPPORTS WITH 10D GALVANIZED NAILS AT 12 INCHES ON CENTER. PROVIDE 2X4 BLOCKING ALONG ALL PANEL EDGES WHERE SHOWN ON THE DRAWINGS. FASTEN ROOF SHEATHING TO BLOCKING OVER EXTERIOR WALLS WITH 10D GALVANIZED NAILS AT 4" O/C.

WALL SHEATHING: EXCEPT WHERE NOTED OTHERWISE, WALL SHEATHING SHALL BE 15/32" THICK STRUCTURAL 1 PLYWOOD WITH EXTERIOR GLUE AND SHALL BE FASTENED TO FRAMING WITH 8D (0.131"x2.5" COMMON, 0.113"x2.5" GALVANIZED BOX) GALV NAILS @ 3" O/C ALONG PANEL EDGES AND 8D GALV NAILS @ 12" O/C ALONG INTERMEDIATE FRAMING. WALL SHEATHING SHALL BE BLOCKED AT ALL EDGES WITH NOMINAL 2" SOLID BLOCKING.

PLYWOOD SIDING: EXCEPT WHERE NOTED OTHERWISE, WALL SIDING SHALL BE 5/8" THICK APA 303 SIDING, DOUGLAS FIR, TEXTURE 1-11 WITH GROOVES 8 INCHES ON CENTER. SIDING SHALL BE FASTENED TO FRAMING WITH 8D (0.131"x2.5" COMMON, 0.113"x2.5" GALVANIZED BOX) GALV NAILS @ 6" O/C ALONG PANEL EDGES AND 8D GALV NAILS @ 12" O/C ALONG INTERMEDIATE FRAMING. SIDING SHALL BE BLOCKED AT ALL EDGES WITH NOMINAL 2" SOLID BLOCKING. OWNER WILL PAINT.

STRUCTURAL STEEL AND CONNECTORS

STRUCTURAL STEEL SHALL CONFORM TO IBC CHAPTER 22, FOR ASTM SPECIFICATION A-36, FY = 36 K.S.I. EXCEPT WHERE NOTED OTHERWISE. STEEL W-SHAPES SHALL CONFORM TO ASTM A992 FY = 50 KSI. STEEL TUBING (TS) SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 K.S.I. DESIGN, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE IBC CHAPTER 22, DIVISION IX, ALLOWABLE STRESS DESIGN. MACHINE BOLTS (MB) SHALL CONFORM TO ASTM 307 UNLESS NOTED OTHERWISE AND SHALL BE PROVIDED WITH STANDARD HEX HEAD NUTS, AND WASHERS CONFORMING TO TABLE 2.

TABLE 2		
BOLT	NUT	WASHER
A307	A563 GR. A	F436 TYPE 1
A325	A563 GR. C	F436 TYPE 1
A490	A563 GR. DH	F436 TYPE 1

TO ASTM A563, GRADE A AND HARDENED STEEL CIRCULAR WASHERS CONFORMING TO ASTM F436. ALL WELDING SHALL CONFORM TO AMERICAN WELDING SOCIETY D1.1. WELD ALL FAYING SURFACES WITH CONTINUOUS 3/16" FILLET WELD (MINIMUM) UNLESS OTHERWISE NOTED. ELECTRODES SHALL BE A.W.S. E-70. ANCHOR ALL COLUMNS WITH MINIMUM (4) 3/4" X 10" ANCHOR BOLTS UNLESS SHOWN OTHERWISE. PROVIDE ADEQUATE LATERAL BRACING FOR STRUCTURE DURING CONSTRUCTION.

STEEL ROOFING PANELS OVERLAP SEAM:

ROOF PANELS SHALL BE 29 GAUGE CORRUGATED STEEL, 36" COVERAGE WITH 5/8" HIGH TRAPEZOIDAL RIDGES AT 9" O/C. PANELS SHALL HAVE A MINIMUM SECTION PROPERTIES AS FOLLOWS: TOP COMPRESSION I_{xx}=0.067 IN⁴/FT, S_{xx}=0.134 IN³/FT, BOTTOM IN COMPRESSION I_{xx}=0.053 IN⁴/FT, S_{xx}=0.137 IN³/FT. MINIMUM STEEL YIELD STRENGTH SHALL BE 80 KSI. PANELS SHALL HAVE A KYMAR 500 FACTORY APPLIED FINISH. PANELS SHALL BE CONNECTED TO EACH SUPPORTING MEMBER WITH #12-14X1 1/4" SELF DRILLING 'TEKS' SCREW IN EACH VALLEY, 5 PER PANEL, AND #1/4-14X7/8" STITCH SCREWS AT 12" O/C ALONG PANEL EDGES. SPECIFIED PANELS ARE 'METAL SALES CLASSIC RIB' CONFORMING TO IBC CLASS A, UL 790

ASPHALT ROOF SHINGLES:

ROOF SHINGLES SHALL BE STANDAR 3 TAB, 235 LB /100SF 'MALARKEY LEGACY' OR APPROVED EQUAL. INSTALL PER MANUFACTURER' WRITTEN 'SHINGLE NSTALLATION INSTRUCTIONS.'

ORIENTED STRAND BOARD (OSB)

ALL ORIENTED STRAND BOARD SHALL CONFORM TO UBC STANDARD 23-3 AND SHALL BE AMERICAN PLYWOOD ASSOCIATION GRADE TRADE MARKED. ALL PANELS SHALL BE NOMINAL 4' X 8' PANELS. UTILIZE FULL SHEETS WHEREVER POSSIBLE. LAY STRENGTH AXIS OF ROOF AND FLOOR SHEATHING PANELS PERPENDICULAR TO SUPPORTS AND WITH PANEL CONTINUOUS OVER THREE OR MORE SPANS. STAGGER END JOINTS OF SUCCESSIVE COURSES 4'.

OSB ROOF SHEATHING: SHALL BE 5/8" PANELS WITH EXTERIOR GLUE AND SHALL BEAR A PANEL IDENTIFICATION INDEX OF 40/20. ROOF SHEATHING SHALL BE FASTENED TO SUPPORTS WITH 10D GALVANIZED NAILS SPACED AT 6" O/C ALONG PANEL EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE FRAMING. PROVIDE 2X BLOCKING ALONG ALL PANEL EDGES WHERE SHOWN ON THE DRAWINGS. FASTEN ROOF SHEATHING TO BLOCKING OVER SHEAR WALLS AND EXTERIOR WALLS WITH 10D GALVANIZED NAILS AT 3" O/C.

OSB WALL SHEATHING: SHALL BE 7/16" PANELS WITH EXTERIOR GLUE. UNLESS NOTED OTHERWISE ON THE DRAWINGS, WALL SHEATHING SHALL BE FASTENED TO FRAMING WITH 8D GALVANIZED NAILS 3" O/C ALONG PANEL EDGES AND 12" O/C ALONG INTERMEDIATE SUPPORTS. WALL SHEATHING SHALL BE BLOCKED AT ALL EDGES WITH NOMINAL 2" SOLID BLOCKING.

MANUFACTURED WOOD JOISTS

FABRICATED WOOD JOISTS SHALL BE OF THE SIZE INDICATED ON THE DRAWINGS AND AS MANUFACTURED BY BOISE CASCADE OR APPROVED EQUAL. FOLLOW MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION, HANDLING AND ERECTION OF FLOOR JOISTS. PROVIDE WEB STIFFENERS PER MANUFACTURER'S RECOMMENDATIONS AND ON BOTH SIDES OF WEBS WHERE JOISTS ARE SUPPORTED BY JOIST HANGERS. PROVIDE BLOCKING PANELS AT SUPPORTS AND ENDS OF JOISTS. PROVIDE FULL THICKNESS BACKER PLATE BETWEEN DOUBLE JOISTS WHERE JOIST IS FRAMED PERPENDICULAR INTO A DOUBLE JOIST. PROVIDE SOLID RIM JOISTS AROUND FULL PERIMETER OF FLOOR SYSTEM. UNLESS NOTED OTHERWISE ON THE DRAWINGS, PROVIDE LAMINATED VENEER LUMBER HEADERS, WITH DEPTH EQUAL TO JOIST DEPTH, AT ALL OPENINGS IN FLOOR AND ROOF SYSTEMS. UNLESS NOTED OTHERWISE, PROVIDE FACE MOUNT JOIST HANGERS WITH CAPACITY EQUAL TO THE FULL SHEAR CAPACITY OF THE JOIST AT ALL JOISTS WHERE JOIST IS NOT SUPPORTED BY DIRECT BEARING.

WINDOW PANELS:

'SUNTUF' CLEAR POLYCARBONATE PANELS NOMINAL 26" WIDTH



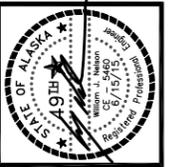
NO.	REVISION	DATE
1	DOUBLE HELIX PILES	7/07/16

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PUBLIC WORKS STORAGE SHELTER
CITY OF HOMER
STRUCTURAL NOTES

PROJECT NO. 1523
DRAWN BY: WJN, CM
CHECKED BY: WJN
DATE: 06/15/2015
SCALES: NOTED
HORIZ. NOTED
VERT. NOTED
SHEET S2
2 OF 13



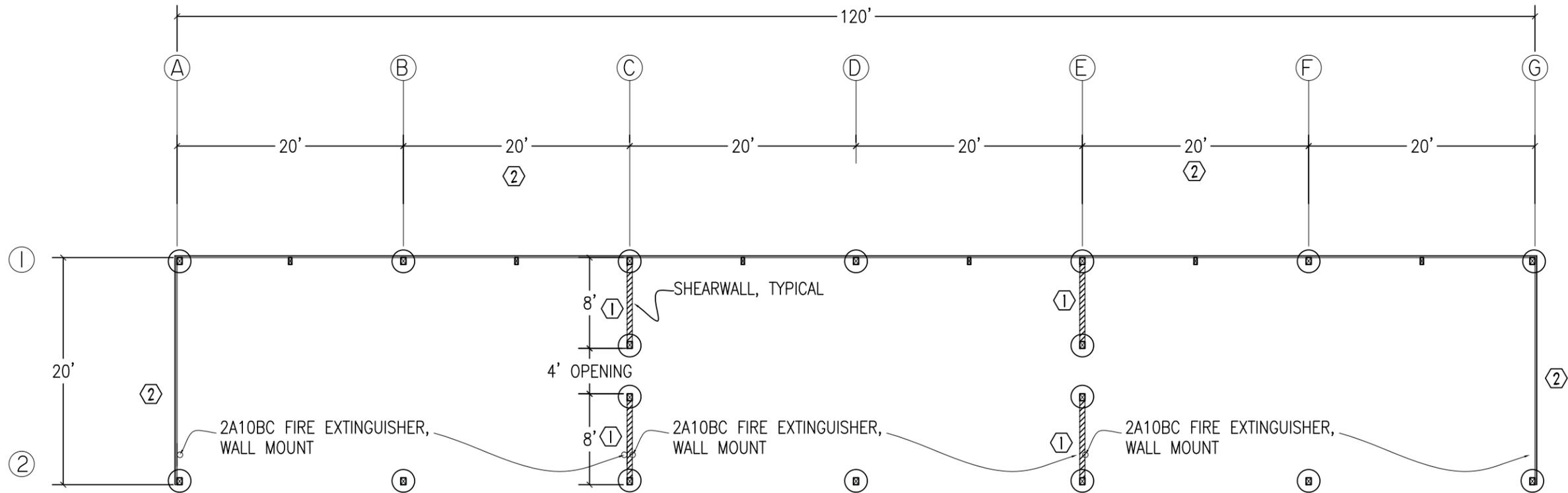
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SHEAR WALL SCHEDULE

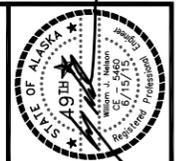
MARK	DESCRIPTION
①	1/2" CDX 8d@3"OC EDGES, 12"O/C FIELD
②	STEEL PANEL -OR- T1-11 SIDING WITH 8D @ 4" O/C EDGES 12" O/C FIELD



A
5 **FLOOR PLAN**
SCALE: 3/16" = 1' (22X34 PLOT) 3/32" = 1' (11X17 PLOT)

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CITY OF HOMER
FLOOR PLAN

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WJN
DATE: 06/15/2015
SCALES: NOTED
HORIZ. NOTED
VERT. NOTED
SHEET **S5**
5 OF 13



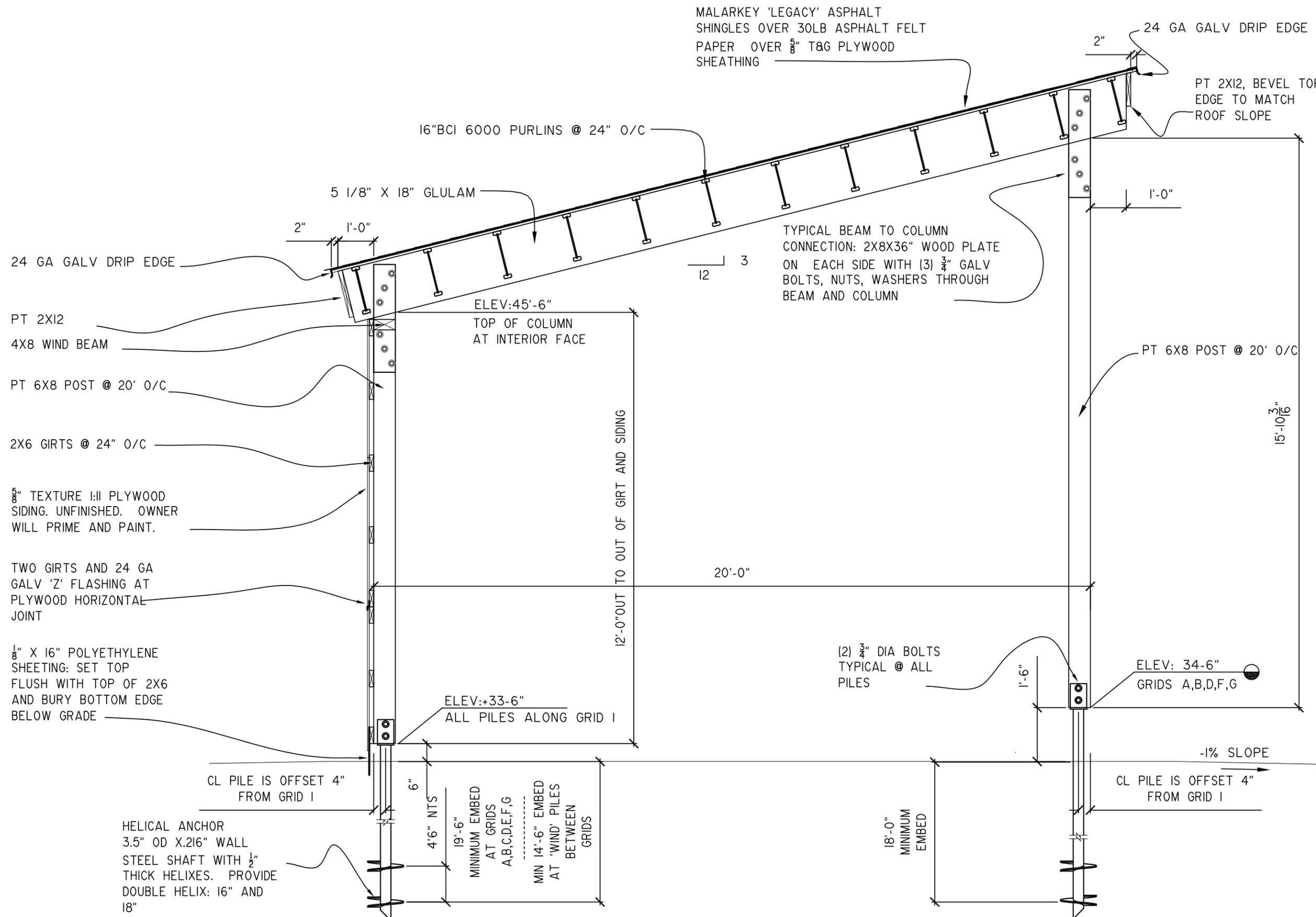
NO.	REVISION	DATE
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PUBLIC WORKS STORAGE SHELTER
CITY OF HOMER
TYPICAL TRANSVERSE SECTION

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SCALES: NOTED
HORIZ. NOTED
VERT. NOTED
SHEET **S7**
7 OF 13



A
7 TYPICAL TRANSVERSE SECTION
SCALE: 3/4" = 1' (22X34 PLOT) 3/8" = 1' (11X17 PLOT)



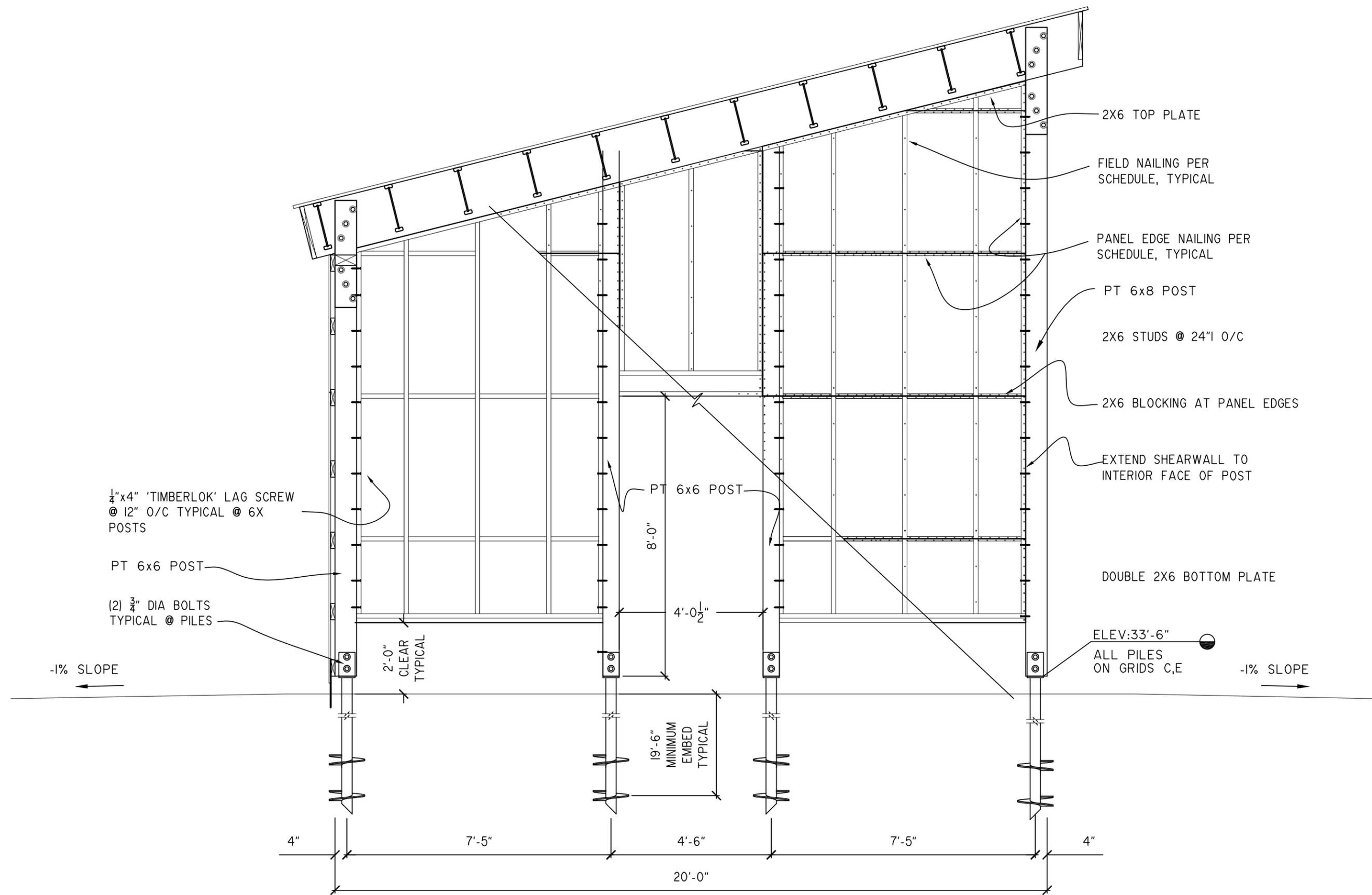
NO.	REVISION	DATE
1	DOUBLE HELIX PILES	7/07/16

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PUBLIC WORKS STORAGE SHELTER
CITY OF HOMER
SECTION AT SHEARWALLS

PROJECT NO. 1523
DRAWN BY: WJN, CM
CHECKED BY: WJN
DATE: 06/15/2015
SCALES: NOTED
HORIZ. NOTED
VERT. NOTED
SHEET **S8**
8 OF 13



A
8 SECTION AT SHEARWALLS
SCALE: 3/4" = 1' (22X34 PLOT) 3/8" = 1' (11X17 PLOT)



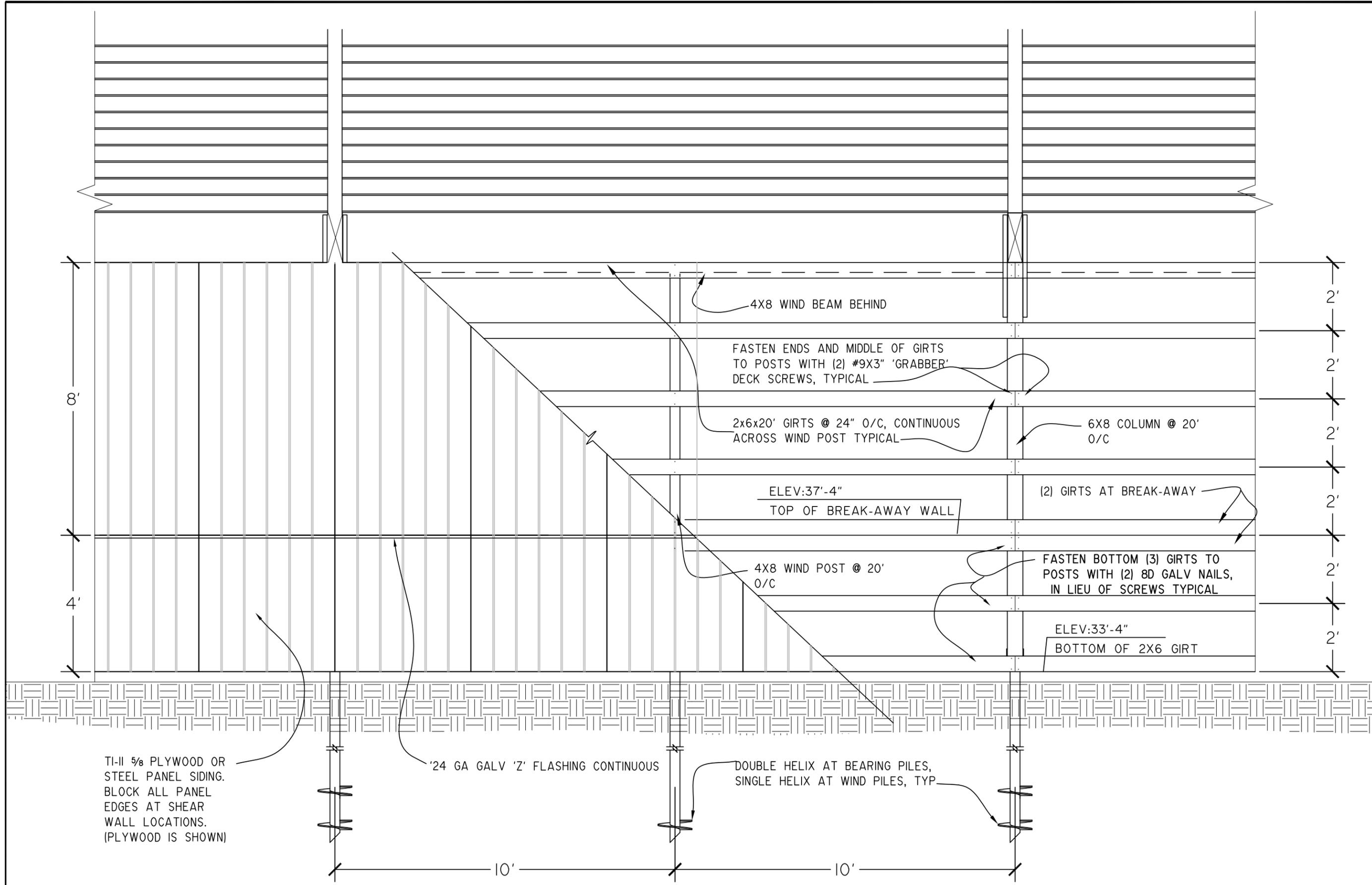
NO.	REVISION	DATE
1	DOUBLE HELIX PILES	7/07/16

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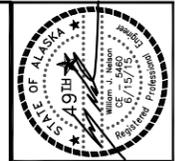


PUBLIC WORKS STORAGE SHELTER
CITY OF HOMER
LONGITUDINAL SECTION AT SHEARWALL

PROJECT NO. 1523
DRAWN BY: WJN, CM
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DATE: 06/15/2015
SCALES: NOTED
HORIZ. NOTED
VERT. NOTED
SHEET **S9**
9 OF 13



A
9 LONGITUDINAL SECTION AT SHEARWALL
SCALE: 3/4" = 1' (22X34 PLOT) 3/8" = 1' (11X17 PLOT)



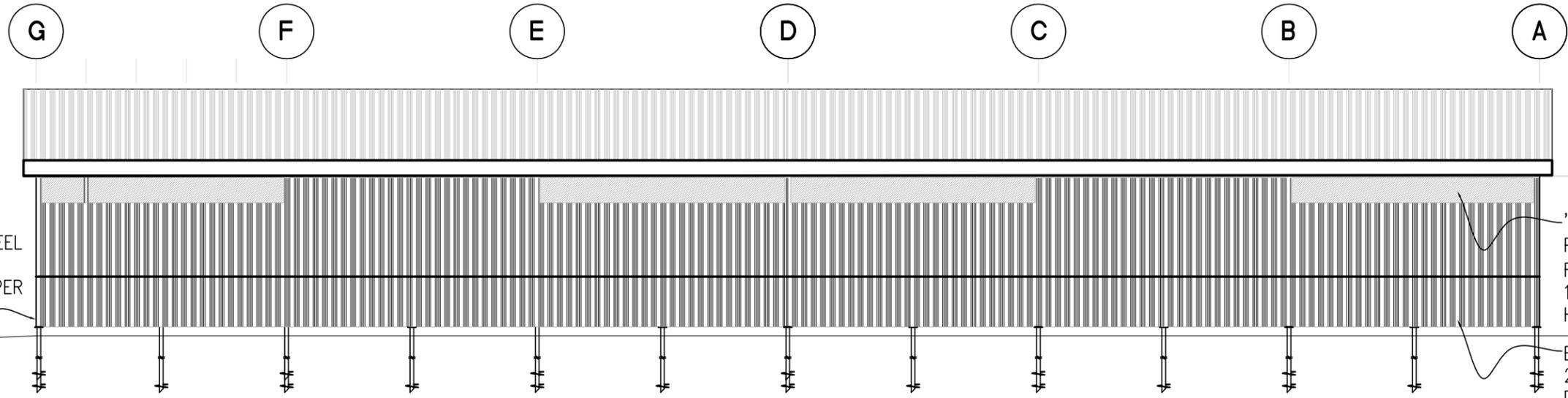
NO.	REVISION	DATE
1	DOUBLE HELIX PILES	7/07/15

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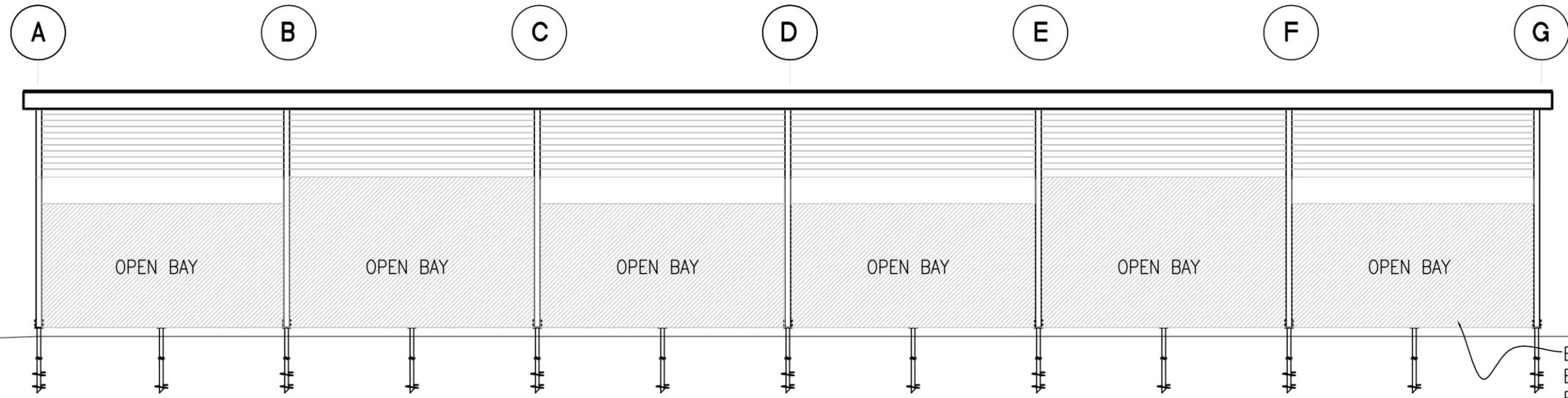


PUBLIC WORKS STORAGE SHELTER
CITY OF HOMER
BUILDING ELEVATIONS

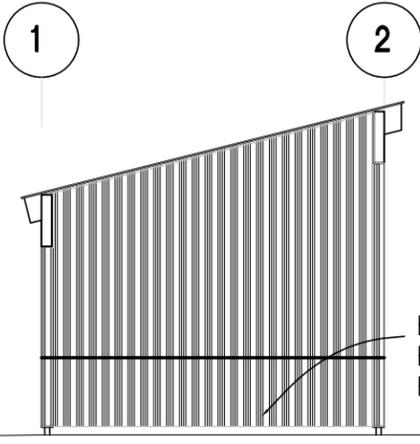
PROJECT NO. 1523
DRAWN BY: WJN, CM
CHECKED BY: WJN
DATE: 06/15/2015
SCALES: NOTED
HORIZ. NOTED
VERT. NOTED
SHEET **S10**
10 OF 13



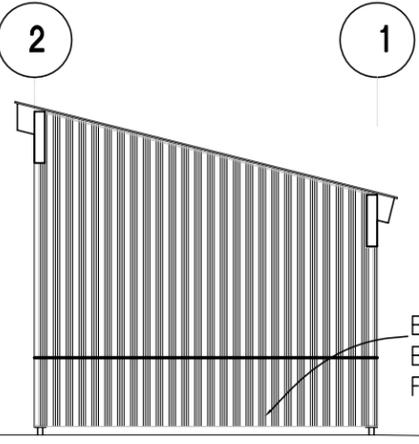
A
10 WEST ELEVATION
SCALE: 3/16" = 1' (22X34 PLOT) 3/32" = 1' (11X17 PLOT)



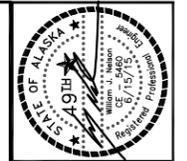
B
10 EAST ELEVATION
SCALE: 3/16" = 1' (22X34 PLOT) 3/32" = 1' (11X17 PLOT)



C
10 SOUTH ELEVATION
SCALE: 3/16" = 1' (22X34 PLOT) 3/32" = 1' (11X17 PLOT)



D
10 NORTH ELEVATION
SCALE: 3/16" = 1' (22X34 PLOT) 3/32" = 1' (11X17 PLOT)



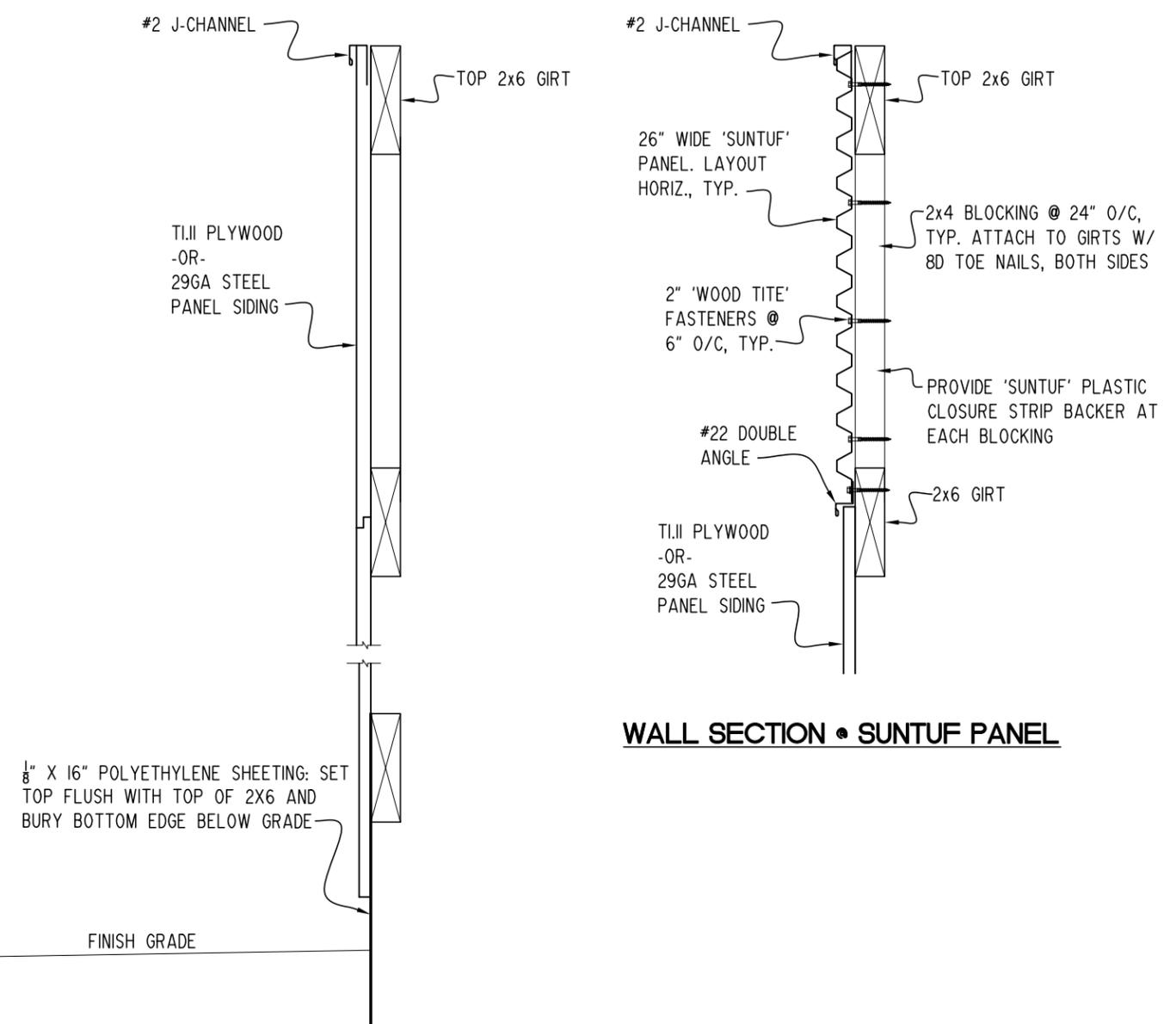
NO.	REVISION	DATE
1	DOUBLE HELIX PILES	7/07/15

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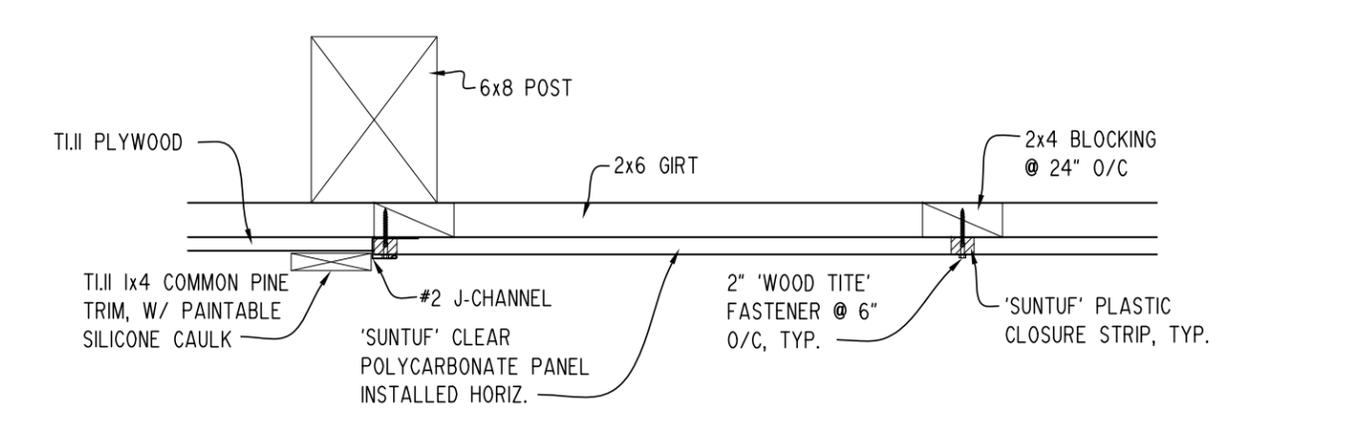
PROJECT NO. 1523
DRAWN BY: WJN, CM
CHECKED BY: WJN
DATE: 06/15/2015
SCALES: NOTED
HORIZ. NOTED
VERT. NOTED
SHEET **S11**
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WALL SECTION @ SUNTUF PANEL

A TYPICAL WALL SECTIONS

SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



B RAKE @ T1.11 PLYWOOD

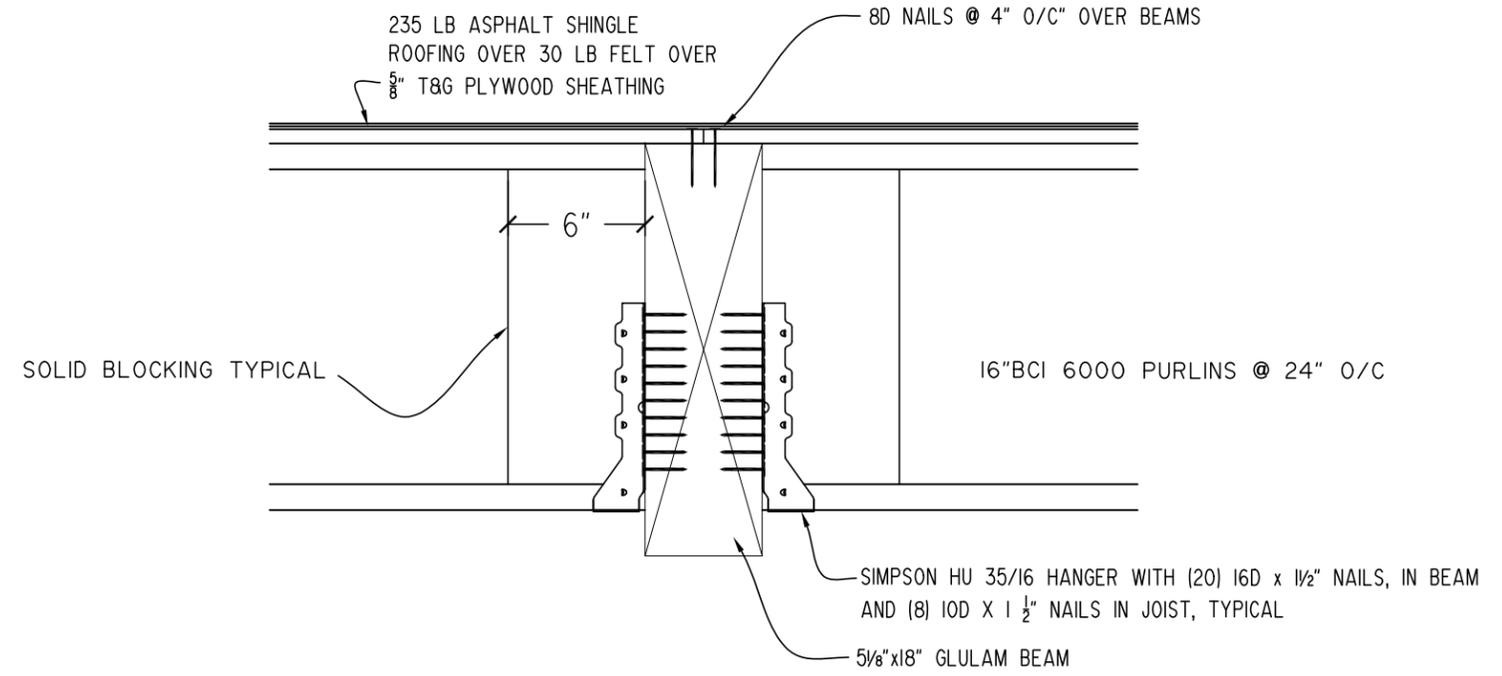
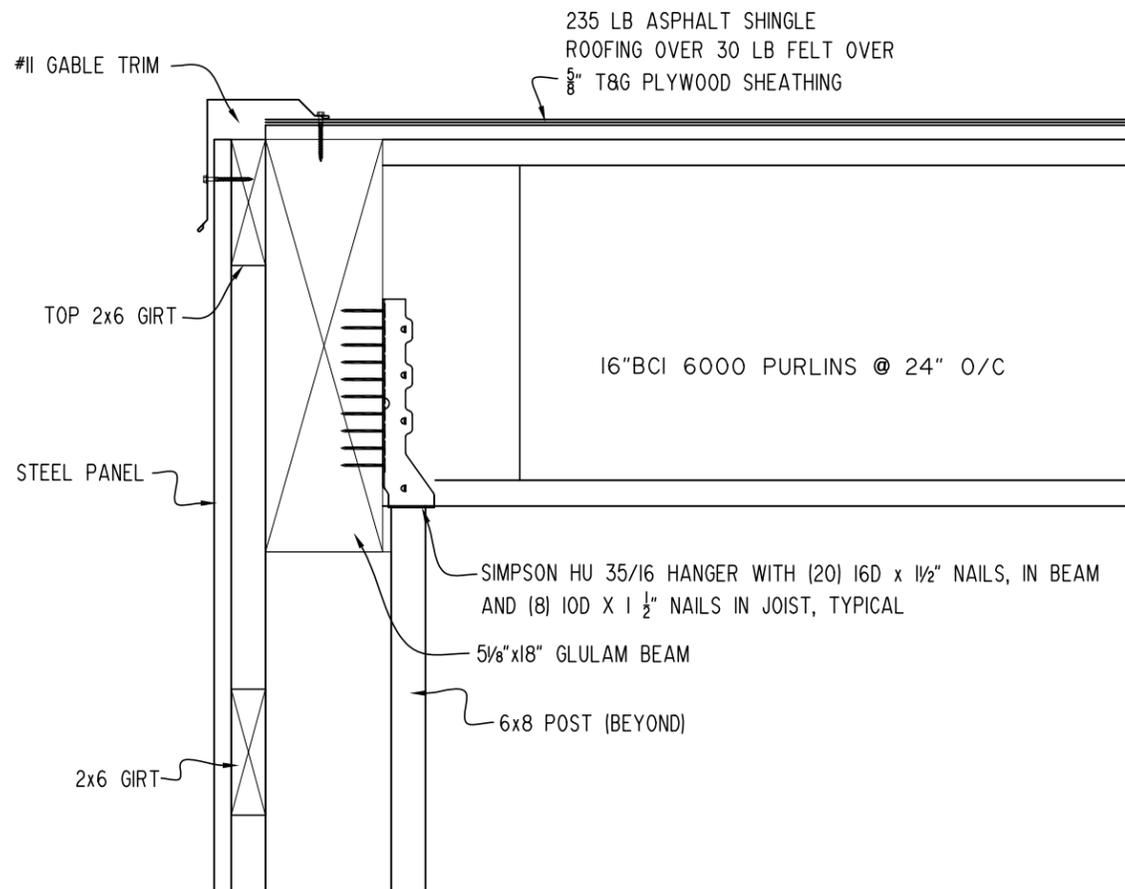
SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)

C JAMB @ SUNTUF WINDOW W/ T1.11 PLYWOOD

SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)

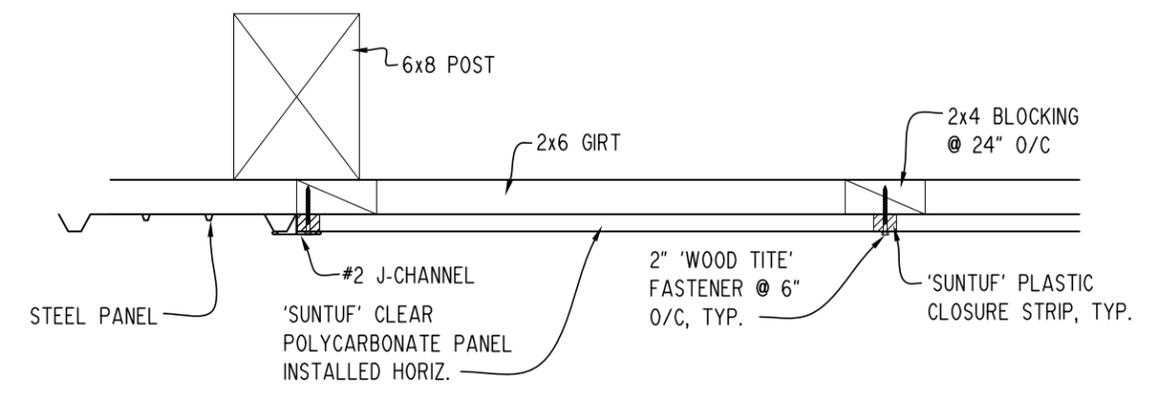
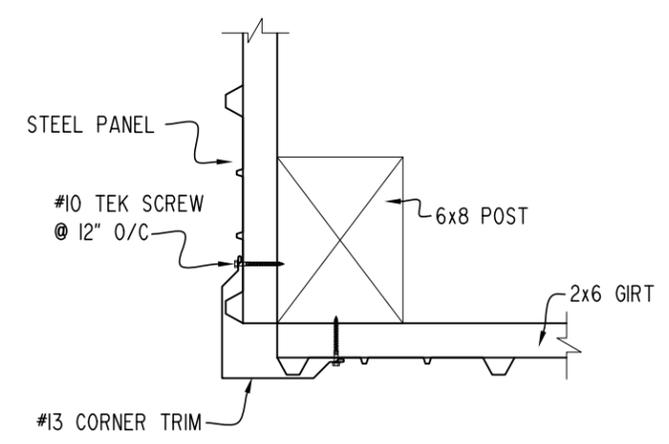
D CORNER ATTACHMENT @ T1.11 PLYWOOD

SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



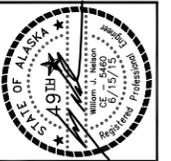
A RAKE @ STEEL PANEL
12 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)

B TYPICAL PURLIN/BEAM CONNECTION
12 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



C CORNER TRIM @ STEEL PANEL
12 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)

D JAMB @ SUNTUF WINDOW W/ STEEL PANEL
12 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



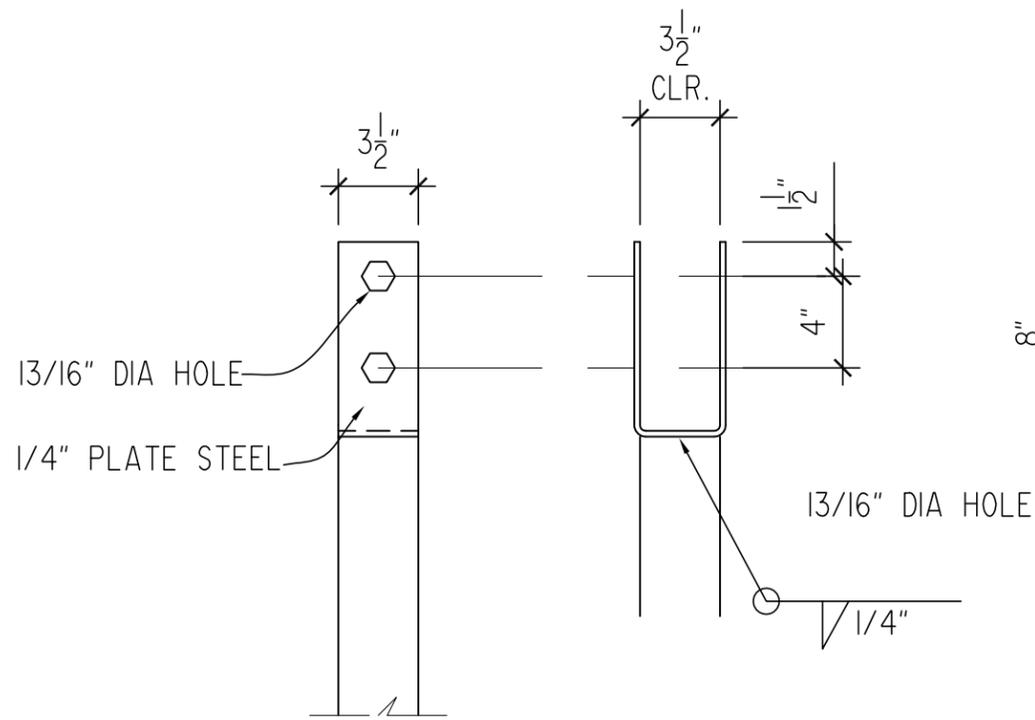
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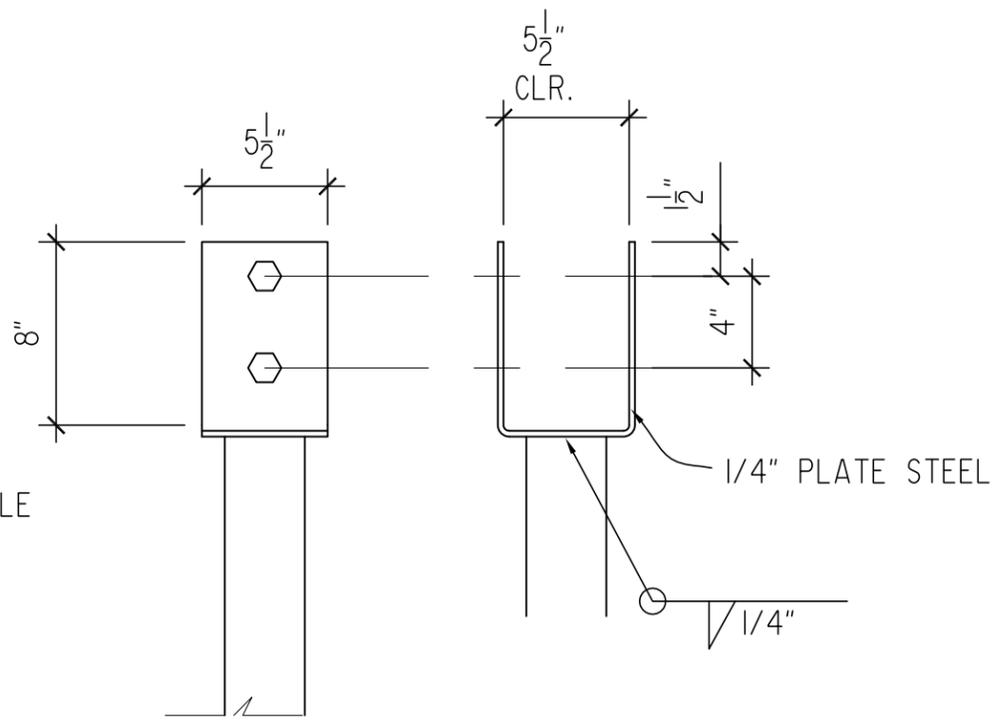
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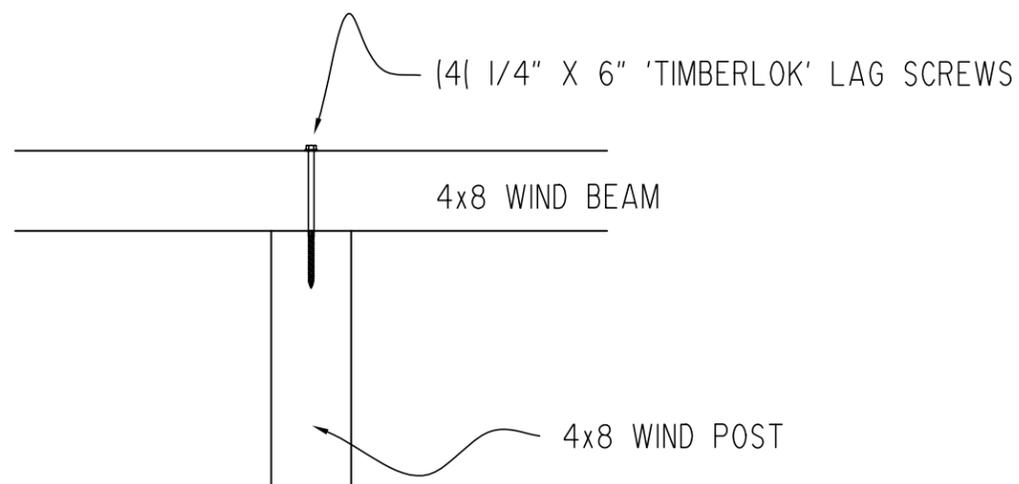
A PIPE CAP AT 4X8

13 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



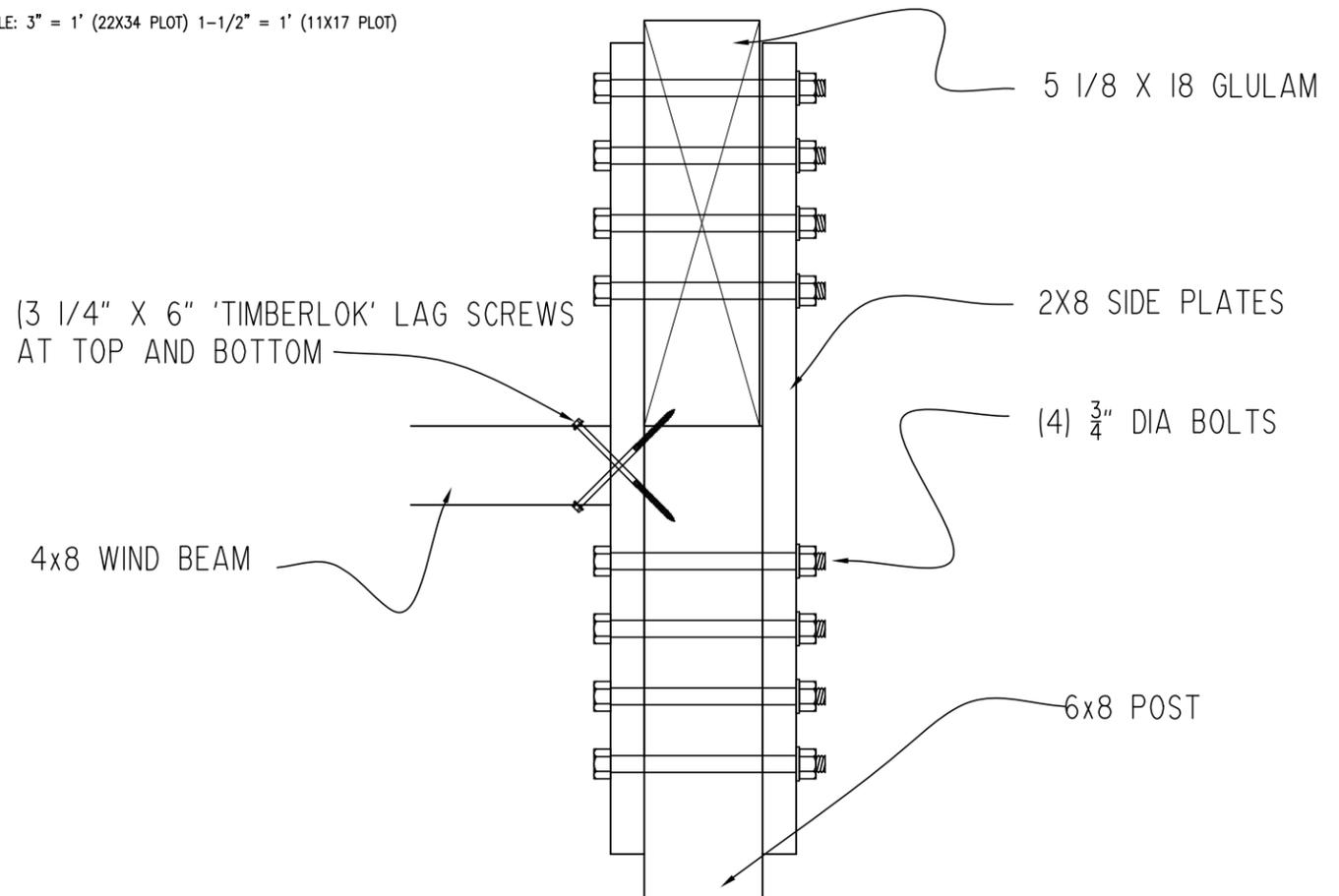
B PILE CAP AT 6X6 OR 6X8

13 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



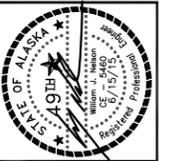
C TYPICAL WIND BEAM/WIND POST CONNECTION

13 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



D TYPICAL BEAM/ POST CONNECTION

13 SCALE: 3" = 1' (22X34 PLOT) 1-1/2" = 1' (11X17 PLOT)



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